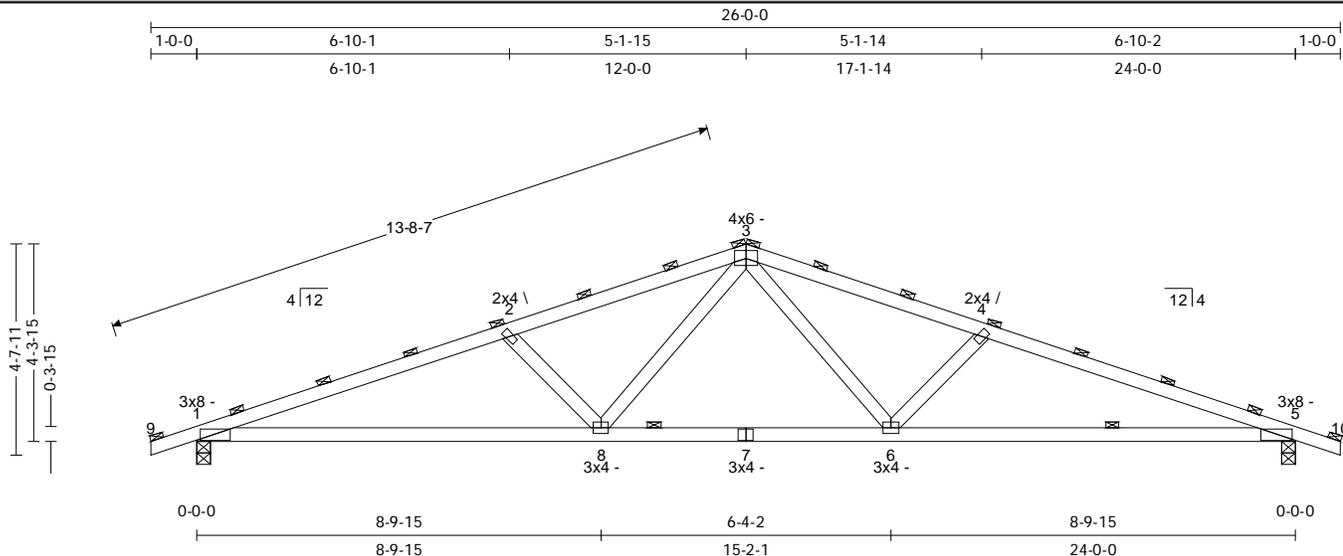


C & M Truss
8319 Ashridge Arnhim
Sardinia, OH 45171
937.446.3400

Truss: 24cAg
Job: 4_12pAG
Designer:Aston Wagner
Date: 02/09/26 09:10:11
Page: 1 of 1

SPAN 24-0-0	PITCH 4/12	QTY 1	OHL 1-0-0	OHR 1-0-0	CANTL 0-0-0	CANTR 0-0-0	PLY(S) 1	SPACING 48 in	WGT/PLY 94 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.91 (4-5)	Vert TL: 0.4 in	L/707	(5-6)	L/180
GSL: 25	TP1 1-2014	BC: 0.79 (5-6)	Vert LL: 0.24 in	L/999	(5-6)	L/240
TCDL: 4	Rep Mbr: No	Web: 0.19 (3-6)	Horz TL: 0.09 in		5	
BCLL: 0	Lumber D.O.L.: :125 %					
BCDL: 4						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.70 in	1,440 lbs	-	-140 lbs	-76 lbs	-140 lbs	8 lbs
5	1	3.5 in	1.70 in	1,440 lbs	-	-140 lbs	-76 lbs	-140 lbs	-

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#2 2 x 4

Bracing

TC: Purlins at 24" OC, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects of balanced (13.2 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. Ventilated. Unobstructed slippery surface. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure B, Enclosed, Gable/Hip, Risk Category I, h = 15 ft, Not End Zone Truss, Both end webs considered. DOL = 1.60
- 4) Minimum storage attic loading has not been applied in accordance with IBC 1607.1
- 5) In accordance with IBC 1607.1, minimum BCLL's do not apply
- 6) This truss is designed as an agricultural truss which for the purposes of this program is defined as a structure that represents a low hazard to people and property See BCSS-10 for installation and temporary bracing.

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.910	320 lbs	(-3,193 lbs)	3-4	0.590		(-2,744 lbs)
	2-3	0.589		(-2,744 lbs)	4-5	0.912	320 lbs	(-3,193 lbs)
BC	5-6	0.793	2,954 lbs		6-8	0.514	2,037 lbs	
Web	2-8	0.125		(-669 lbs)	3-8	0.186	817 lbs	
					3-6	0.186	817 lbs	
					4-6	0.125		(-668 lbs)

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Building Designer shall verify self weight of the truss and other dead load materials do not exceed TCDDL 4 psf.
- 4) Building Designer shall verify self weight of the truss and other dead load materials do not exceed BCDL 4 psf.
- 5) Design assumes minimum x2 (flat orientation, visually graded) purlins attached to the top of the TC at purlin spacing shown with at least 2-10d nails.
- 6) Brace bottom chord with approved sheathing or purlins per Bracing Summary
- 7) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member
- 8) A creep factor of 2.00 has been applied for this truss analysis.
- 9) The "SYP" label shown in the "Material Summary" above indicates the new SP1B design values effective June 1, 2013 were used.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.

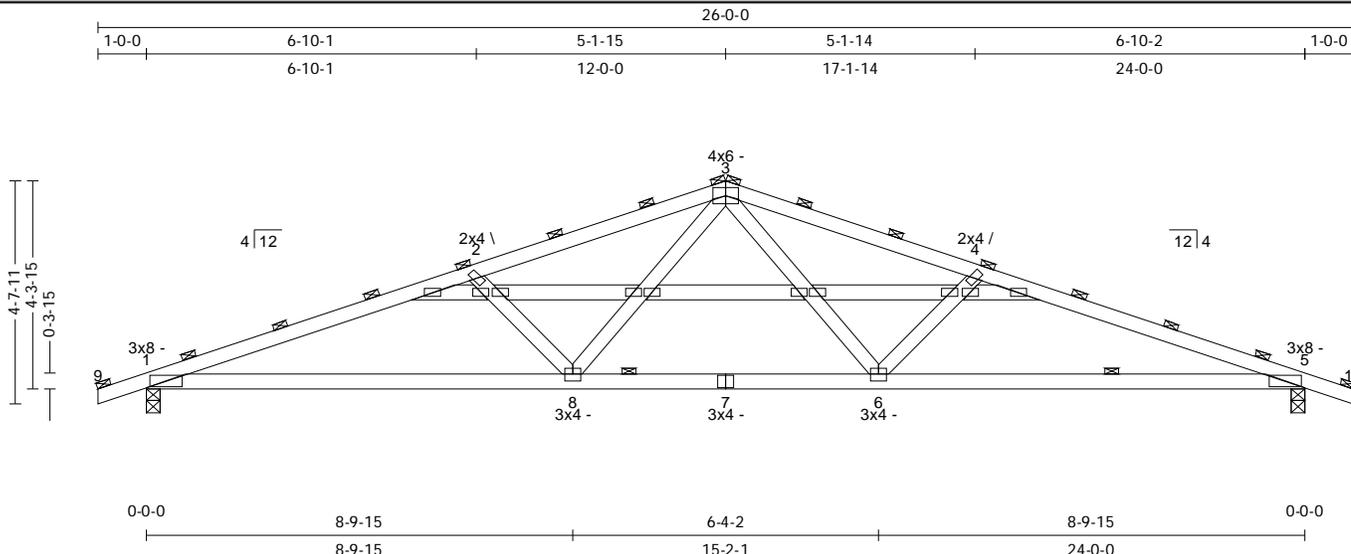
WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software V5.8.14
Eagle Metal Products

C & M Truss
 8319 Ashridge Arnhiem
 Sardinia, OH 45171
 937.446.3400

Truss: 24gAG
 Job: 4_12pAG
 Designer: Aston Wagner
 Date: 02/09/26 09:10:19
 Page: 1 of 1

SPAN 24-0-0	PITCH 4/12	QTY 1	OHL 1-0-0	OHR 1-0-0	CANTL 0-0-0	CANTR 0-0-0	PLY(S) 1	SPACING 48 in	WGT/PLY 113 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

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Reaction

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Material

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Web	2-8	0.125		(-669 lbs)	3-8	0.186	817 lbs		3-6	0.186	817 lbs
									4-6	0.125	(-668 lbs)

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable webs placed at 24" OC, U.N.O.
- Attach structural gable blocks with 2x4 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCS1B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- Building Designer shall verify self weight of the truss and other dead load materials do not exceed TCDDL 4 psf.
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- Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
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