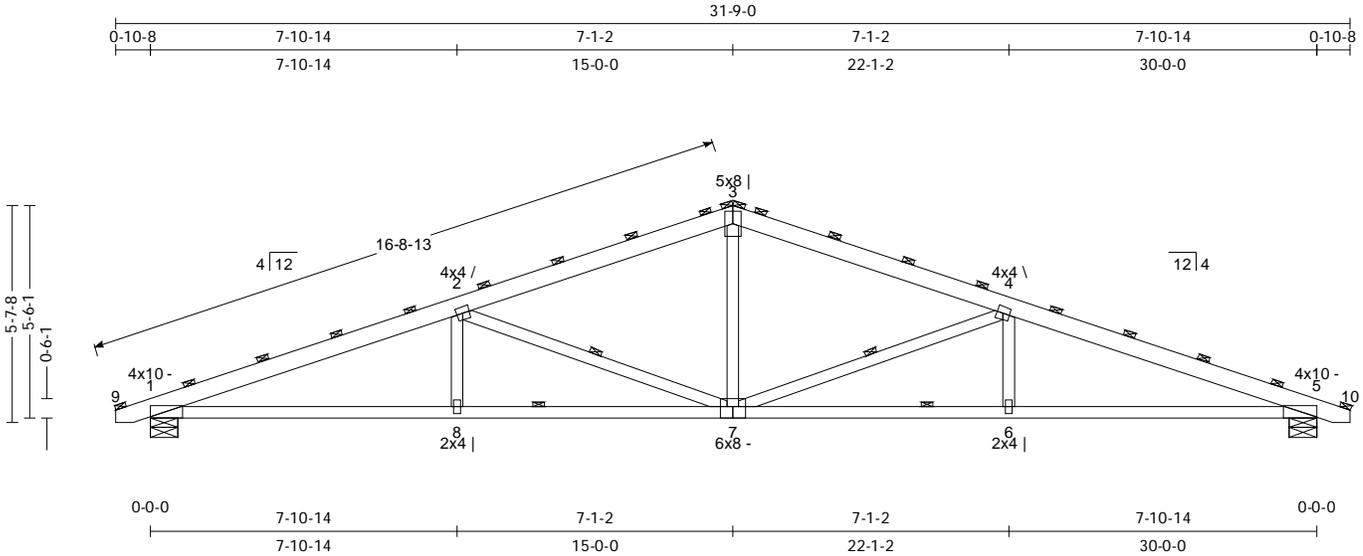


C & M Truss
8319 Ashridge Arnhem
Sardinia, OH 45171
937.446.3400

Truss: 30cAg
Job: 4_12pAG
Designer: Aston Wagner
Date: 02/09/26 09:10:50
Page: 1 of 1

SPAN 30-0-0	PITCH 4/12	QTY 1	OHL 0-10-8	OHR 0-10-8	CANTL 0-0-0	CANTR 0-0-0	PLY(S) 1	SPACING 48 in	WGT/PLY 151 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.53 (1-2)	Vert TL: 0.39 in	L / 890	(6-7)	L / 180
GSL: 25	Termin: TPI 1-2014	BC: 0.73 (8-1)	Vert LL: 0.26 in	L / 999	(6-7)	L / 240
TCCL: 4	Rep Mbr: No	Web: 0.35 (4-7)	Horz TL: 0.15 in		5	
BCLL: 0	Lumber D.O.L.: 125 %					
BCCL: 4						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	8.5 in	2.08 in	1,764 lbs	-	-108 lbs	-71 lbs	-108 lbs	-6 lbs
5	1	8.5 in	2.08 in	1,764 lbs	-	-108 lbs	-71 lbs	-108 lbs	-

Material

TC: SYP#1 2 x 6
BC: SYP#1 2 x 4
Web: SYP#2 2 x 4

Bracing

TC: Purlins at 24" OC, Purlin design by Others.
BC: Sheathed or Purlins at 10'-0", Purlin design by Others.
Web: One Midpoint Row: 2-7, 4-7

Loads

- This truss has been designed for the effects of balanced (13.2 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. Ventilated. Unobstructed slippery surface. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure B, Enclosed, Gable/Hip, Risk Category I, h = 15 ft, Not End Zone Truss, Both end webs considered. DOL = 1.60
- Minimum storage attic loading has not been applied in accordance with IBC 1607.1
- In accordance with IBC 1607.1, minimum BCLL's do not apply
- This truss is designed as an agricultural truss which for the purposes of this program is defined as a structure that represents a low hazard to people and property See BCS1-10 for installation and temporary bracing.

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.534	(-3,972 lbs)	3-4	0.396	(-2,799 lbs)			
BC	5-6	0.729	3,668 lbs	6-7	0.721	3,668 lbs	7-8	0.721	3,668 lbs
Web	2-7	0.351	(-1,289 lbs)	4-7	0.351	(-1,289 lbs)			
	3-7	0.222	984 lbs						

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- Building Designer shall verify self weight of the truss and other dead load materials do not exceed TCCL 4 psf.
- Building Designer shall verify self weight of the truss and other dead load materials do not exceed BCCL 4 psf.
- Design assumes minimum x2 (flat orientation, visually graded) purlins attached to the top of the TC at purlin spacing shown with at least 2-10d nails.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary
- Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSB3 for additional information.
- Listed wind uplift reactions based on MWFRS & C&C loading.

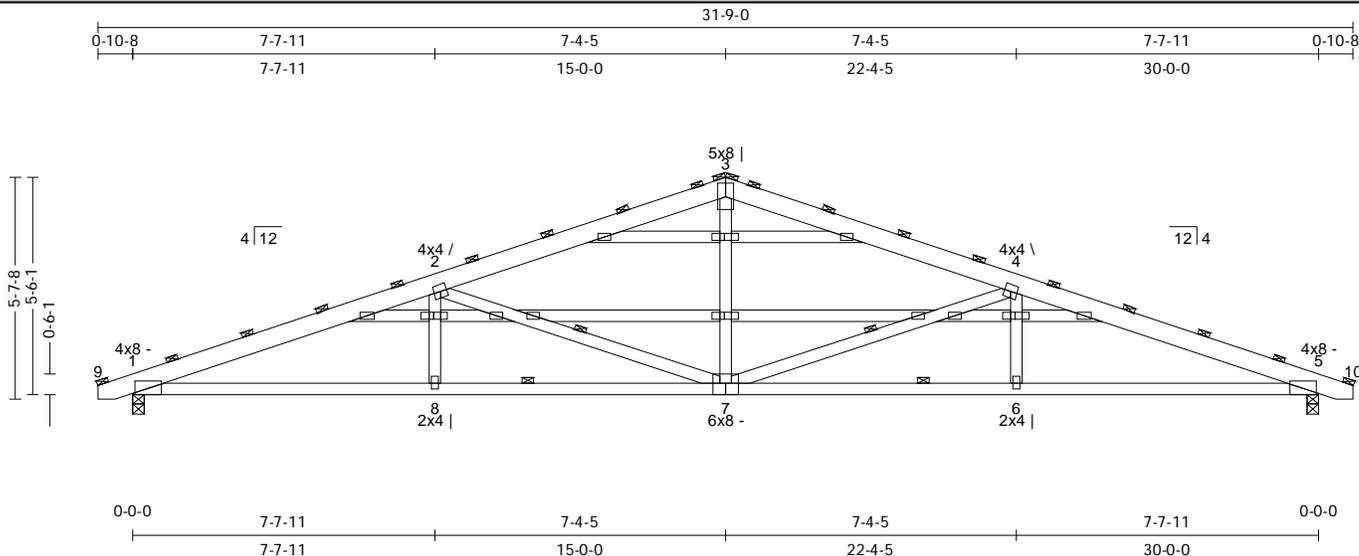
WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software V5.8.14
Eagle Metal Products

C & M Truss
 8319 Ashridge Arnhim
 Sardinia, OH 45171
 937.446.3400

Truss: 30gAG
 Job: 4_12pAG
 Designer:Aston Wagner
 Date: 02/09/26 09:10:56
 Page: 1 of 2

SPAN 30-0-0	PITCH 4/12	QTY 1	OHL 0-10-8	OHR 0-10-8	CANTL 0-0-0	CANTR 0-0-0	PLY(S) 1	SPACING 48 in	WGT/PLY 187 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.49 (4-5)	Vert TL: 0.4 in	L / 891	(6-7)	L / 180
GSL: 25	Termin B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. Ventilated. Unobstructed slippery surface. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.	BC: 0.75 (6-7)	Vert LL: 0.26 in	L / 999	(6-7)	L / 240
TCDL: 4	Rep Mbr: No	Web: 0.37 (4-7)	Horz TL: 0.16 in		5	
BCLL: 0	Lumber D.O.L.: :125 %					
BCDL: 4						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	2.08 in	1,764 lbs	-	-108 lbs	-71 lbs	-108 lbs	-6 lbs
5	1	3.5 in	2.08 in	1,764 lbs	-	-108 lbs	-71 lbs	-108 lbs	-

Material

TC: SYP #1 2 x 6
 BC: SYP #1 2 x 4
 Web: SYP #2 2 x 4

Bracing

TC: Purlins at 24" OC, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
 Web: One Midpoint Row: 2-7, 4-7

Loads

- This truss has been designed for the effects of balanced (13.2 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. Ventilated. Unobstructed slippery surface. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure B, Enclosed, Gable/Hip, Risk Category I, h = 15 ft, Not End Zone Truss, Both end webs considered. DOL = 1.60
- Minimum storage attic loading has not been applied in accordance with IBC 1607.1
- In accordance with IBC 1607.1, minimum BCLL's do not apply
- This truss is designed as an agricultural truss which for the purposes of this program is defined as a structure that represents a low hazard to people and property See BCSE-10 for installation and temporary bracing.

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.494	(-4,000 lbs)	3-4	0.431	(-2,813 lbs)						
	2-3	0.431	(-2,813 lbs)	4-5	0.494	(-4,000 lbs)						
BC	5-6	0.718	3,697 lbs	6-7	0.748	3,697 lbs	7-8	0.748	3,697 lbs	8-1	0.718	3,697 lbs
Web	2-7	0.374	(-1,306 lbs)	4-7	0.374	(-1,306 lbs)						
	3-7	0.214	947 lbs									

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable webs placed at 24" OC, U.N.O.
- Attach structural gable blocks with 2x4 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSEB3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- Building Designer shall verify self weight of the truss and other dead load materials do not exceed TCCLL 4 psf.
- Building Designer shall verify self weight of the truss and other dead load materials do not exceed BCDL 4 psf.
- Design assumes minimum #2 (flat orientation, visually graded) purlins attached to the top of the TC at purlin spacing shown with at least 2-10d nails.
- Gable must be sheathed on one side or lateral bracing applied appropriately
- Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SYP design values effective June 1, 2013 were used.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLY(S)	SPACING	WGT/PLY
30-0-0	4/12	1	0-10-8	0-10-8	0-0-0	0-0-0	1	48 in	187 lbs

- 13) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSB3 for additional information.
14) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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