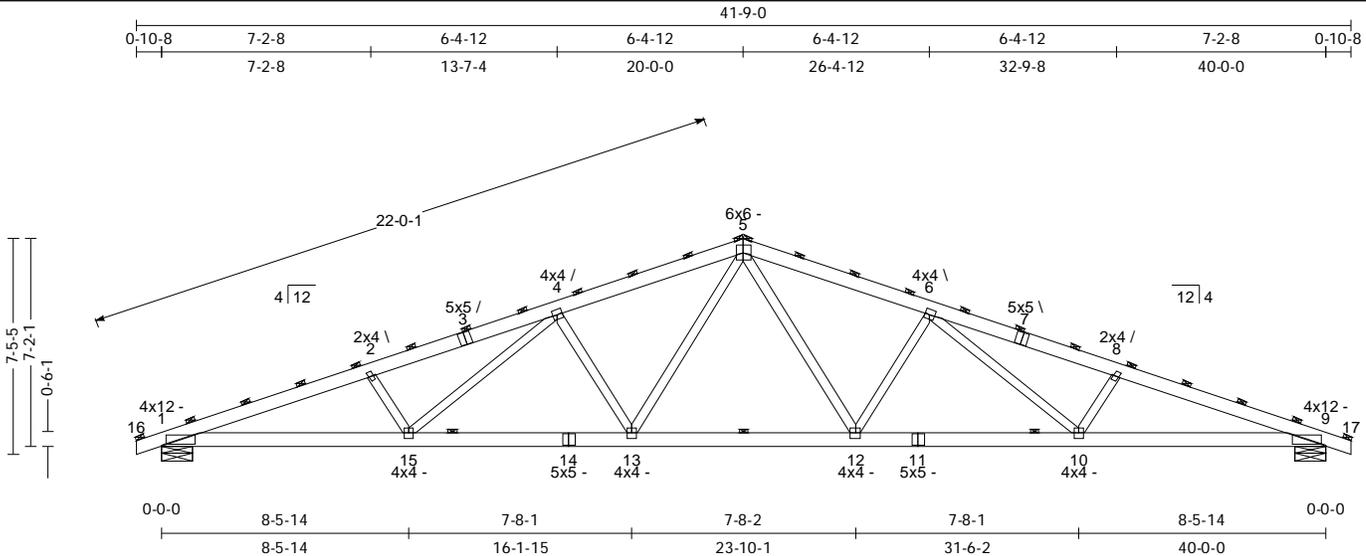


C & M Truss
 8319 Ashridge Arnhem
 Address 2
 Sardinia, OH 45171

Truss: 40cAg
 Job: 4_12pAG
 Designer: Aston Wagner
 Date: 02/09/26 09:11:25
 Page: 1 of 1

SPAN 40-0-0	PITCH 4/12	QTY 1	OHL 0-10-8	OHR 0-10-8	CANTL 0-0-0	CANTR 0-0-0	PLY(S) 1	SPACING 48 in	WGT/PLY 246 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.65 (1-2)	Vet TL: 0.59 in	L/775	(12-13)	L/180
GSL: 25	Terain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. Ventilated. Unobstructed slippery surface. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.	BC: 0.84 (15-1)	Vet LL: 0.4 in	L/999	(12-13)	L/240
TCDL: 5	Rep Mbr: No	Web: 0.49 (6-12)	Horz TL: 0.17 in		9	
BCLL: 0	Lumber D.O.L.: :125 %					
BCDL: 4						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	13 in	2.84 in	2,407 lbs	-	-48 lbs	-	-48 lbs	25 lbs
9	1	13 in	2.84 in	2,407 lbs	-	-48 lbs	-	-48 lbs	-

Material

TC: SYP#1 2 x 6
 BC: SYP#1 2 x 6
 Web: SYP#2 2 x 4

Bracing

TC: Purlins at 24" OC, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (13.2 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. Ventilated. Unobstructed slippery surface. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure B, Enclosed, Gable/Hip, Risk Category I, h = 15 ft, Not End Zone Truss, Both end webs considered. DOL = 1.60
- Minimum storage attic loading has not been applied in accordance with IBC 1607.1
- In accordance with IBC 1607.1, minimum BCLL's do not apply
- This truss is designed as an agricultural truss which for the purposes of this program is defined as a structure that represents a low hazard to people and property See BCSE10 for installation and temporary bracing.

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.654	(-6,071 lbs)	4-5	0.403	(-4,417 lbs)	6-8	0.527	(-5,748 lbs)
	2-4	0.525	(-5,748 lbs)	5-6	0.403	(-4,417 lbs)	8-9	0.654	(-6,071 lbs)
BC	9-10	0.839	5,677 lbs	12-13	0.452	3,506 lbs	15-1	0.839	5,677 lbs
	10-12	0.603	4,616 lbs	13-15	0.603	4,616 lbs			
Web	2-15	0.095	(-631 lbs)	4-13	0.494	(-1,077 lbs)	5-12	0.275	1,220 lbs
	4-15	0.231	1,025 lbs	5-13	0.275	1,220 lbs	6-12	0.494	(-1,077 lbs)
							6-10	0.231	1,025 lbs
							8-10	0.095	(-631 lbs)

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- Building Designer shall verify self weight of the truss and other dead load materials do not exceed TCCLL 5 psf.
- Building Designer shall verify self weight of the truss and other dead load materials do not exceed BCDL 4 psf.
- Design assumes minimum x2 (flat orientation, visually graded) purlins attached to the top of the TC at purlin spacing shown with at least 2-10d nails.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary
- Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Listed wind uplift reactions based on MWFRS & C&C loading.

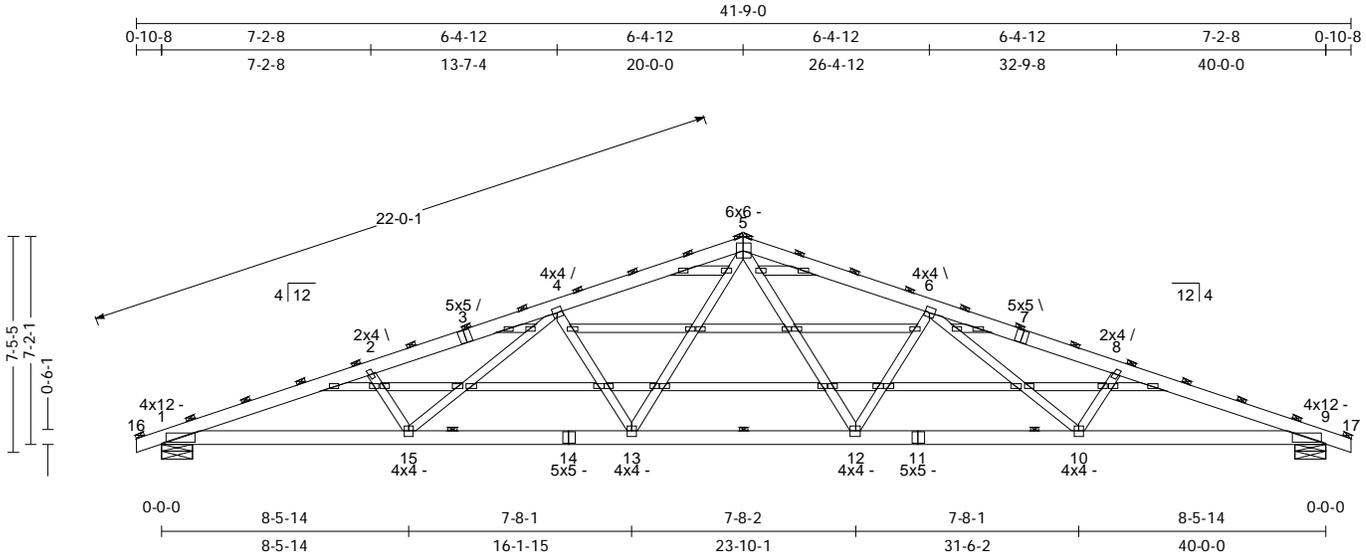
WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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 Eagle Metal Products

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8319 Ashridge Arnhem
Address 2
Sardinia, OH 45171

Truss: 40gAg
Job: 4_12pAG
Designer: Aston Wagner
Date: 02/09/26 09:11:30
Page: 1 of 2

SPAN 40-0-0	PITCH 4/12	QTY 1	OHL 0-10-8	OHR 0-10-8	CANTL 0-0-0	CANTR 0-0-0	PLY(S) 1	SPACING 48 in	WGT/PLY 315 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.65 (1-2)	Vert TL: 0.59 in	L/775	(12-13)	L/180
GSL: 25	TPI 1-2014	BC: 0.84 (15-1)	Vert LL: 0.4 in	L/999	(12-13)	L/240
TCDL: 5	Rep Mbr: No	Web: 0.49 (6-12)	Horz TL: 0.17 in		9	
BCLL: 0	Lumber D.O.L.: 125 %					
BCDL: 4						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	13 in	2.84 in	2,407 lbs		-48 lbs		-48 lbs	25 lbs
9	1	13 in	2.84 in	2,407 lbs		-48 lbs		-48 lbs	

Material

TC: SYP#1 2 x 6
BC: SYP#1 2 x 6
Web: SYP#2 2 x 4

Bracing

TC: Purlins at 24" OC, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (13.2 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.10), DOL = 1.15. Ventilated. Unobstructed slippery surface. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure B, Enclosed, Gable/Hip, Risk Category I, h = 15 ft, Not End Zone Truss, Both end webs considered. DOL = 1.60
- Minimum storage attic loading has not been applied in accordance with IBC 1607.1
- In accordance with IBC 1607.1, minimum BCLL's do not apply
- This truss is designed as an agricultural truss which for the purposes of this program is defined as a structure that represents a low hazard to people and property See BCSE-10 for installation and temporary bracing.

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.654	(-6,071 lbs)	4-5	0.403	(-4,417 lbs)	6-8	0.527	(-5,748 lbs)
	2-4	0.525	(-5,748 lbs)	5-6	0.403	(-4,417 lbs)	8-9	0.654	(-6,071 lbs)
BC	9-10	0.839	5,677 lbs	12-13	0.452	3,506 lbs	15-1	0.839	5,677 lbs
	10-12	0.603	4,616 lbs	13-15	0.603	4,616 lbs			
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	4-15	0.231	1,025 lbs	5-13	0.275	1,220 lbs	6-12	0.494	(-1,077 lbs)
							6-10	0.231	1,025 lbs
							8-10	0.095	(-631 lbs)

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable webs placed at 24" OC, U.N.O.
- Attach structural gable blocks with 2x4 2ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSEB3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- Building Designer shall verify self weight of the truss and other dead load materials do not exceed TCDDL 5 psf.
- Building Designer shall verify self weight of the truss and other dead load materials do not exceed BCDL 4 psf.
- Design assumes minimum x2 (flat orientation, visually graded) purlins attached to the top of the TC at purlin spacing shown with at least 2-10d nails.
- Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP#2" label shown in the "Material Summary" above indicates the new SDR design values effective June 1, 2013 were used.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLY(S)	SPACING	WGT/PLY
40-0-0	4/12	1	0-10-8	0-10-8	0-0-0	0-0-0	1	48 in	315 lbs

12) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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